

EARTHQUAKE ANALYSIS FEATURES OF BUILD-MASTER 2005

We have recently released New version of **BUILD-MASTER** i.e. **Version 2005**. Many new features are added in the software. The most important revision is that now Earthquake loads are calculated as per **IS 1893 : 2002** code and Wind loads are calculated as per **IS 875 : 1987** code of practice.

In this new version program generates 11 load cases and up to 205 load combinations automatically during the Analysis of Space Frame with Horizontal Loads.

Analysis output includes **factored values** of forces and moments (after multiplying partial safety factors for various combinations), if frame analysis is performed for **Limit State of Collapse**. Alternatively these values are un-factored if **Limit State of Serviceability** option is selected for the analysis. Factored values are used for designing the RCC components and un-factored values are used for checking the deflections and calculating the sizes of footings.

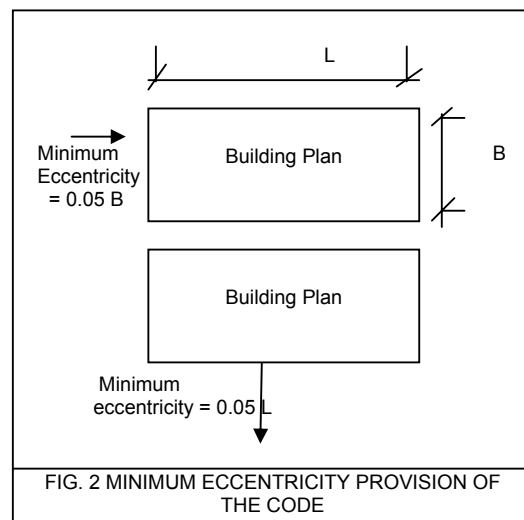
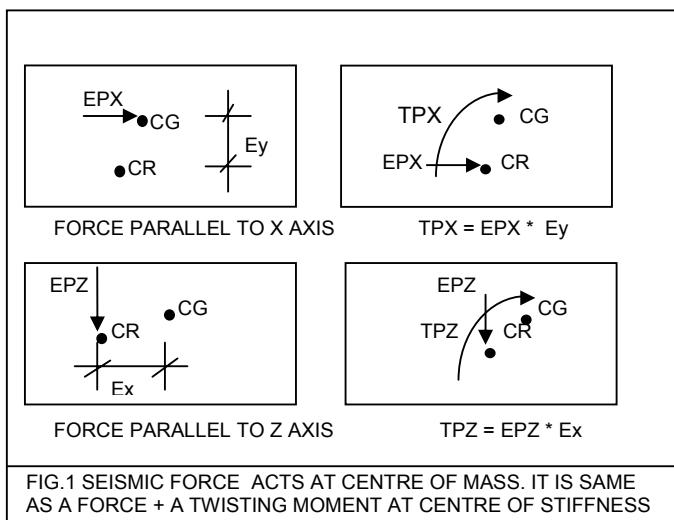
Program calculates the **Center of Gravity** (i.e. center of mass) and **Center of Rotation** (i.e. center of stiffness) at each floor level and considers the design eccentricity while applying the Earthquake forces. Since wind forces act on the face of building, program calculates the **Center of Width** (i.e. center of sides) of each floor while applying the wind loads.

CALCULATING SEISMIC FORCES:

Torsion:

The seismic force is caused by inertia of the building mass; hence the resultant of seismic forces on each floor acts at the center of mass (i.e. Center of Gravity) **C.G.** of that floor. If the building is not symmetrical about the two principal axes then, the center of mass does not coincide with the center of stiffness. In this case the lateral force at center of mass causes a torsion i.e. it tries to rotate the floor about the center of stiffness (i.e. center of rotation) **C.R.** The lateral force at the **C.G.** can be thought of as lateral force at the **C.R.** plus a twisting moment about vertical Y-axis as illustrated in Fig. 1 below. **EPX** is the earthquake force parallel to X-axis and **EPZ** parallel to Z-axis. E_x is the actual eccentricity between C.G. and C.R. parallel to X axis and E_y parallel to Y-axis. **TPX** is the torsion moment due to EPX and **TPZ** due to EPZ.

Since the calculation of the location of center of mass and center of stiffness, and therefore of eccentricity, is only approximate, the code requires that minimum eccentricity of at least **5%** of the base dimension perpendicular to the direction of applied force as shown in Fig. 2 below shall be considered to account for the "**accidental eccentricity**".



Design Eccentricity (Ed) :

Code specifies Two Cases of Design eccentricity to be used instead of the actual eccentricity while calculating the torsion moment, namely,

Case 1. $E_d = 1.5 * E_a + 0.05 b$ and **Case 2. $E_d = E_a - 0.05 b$**

Where E_a is actual eccentricity, b is the fl. plan dimension perpendicular to the direction of force. Program calculates the Torsion Moment due to design eccentricity for both these cases.

Non Orthogonal Buildings:

Code specifies that when the lateral load resisting elements (beams & columns) are not oriented along the orthogonal horizontal directions (as in the sample building plan shown on the next page), the structure shall be designed for the effect due to full design earthquake load in one horizontal direction plus 30% of the design load in the other direction.

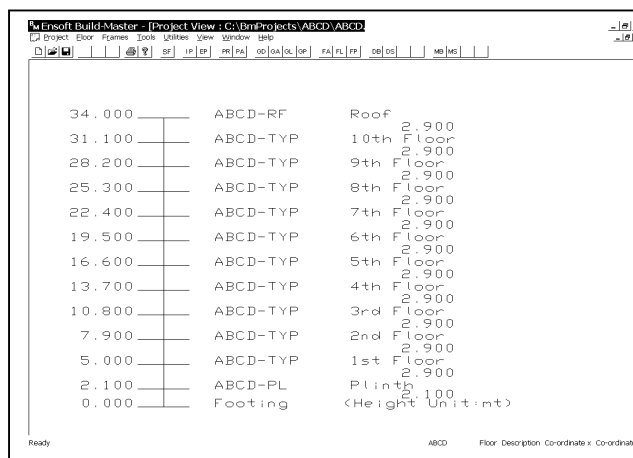
Program takes into account all the above provisions of IS 1893-2002 code while considering the effect of earthquake loads. It gives a option for the user Whether To **Consider Earthquake Eccentricity Effect?** And also asks whether to **Consider the Non-Orthogonal Building Effect?** Generates the appropriate Loading Cases for the analysis.

SPACE FRAME ANALYSIS WITH HORIZONTAL LOADS:

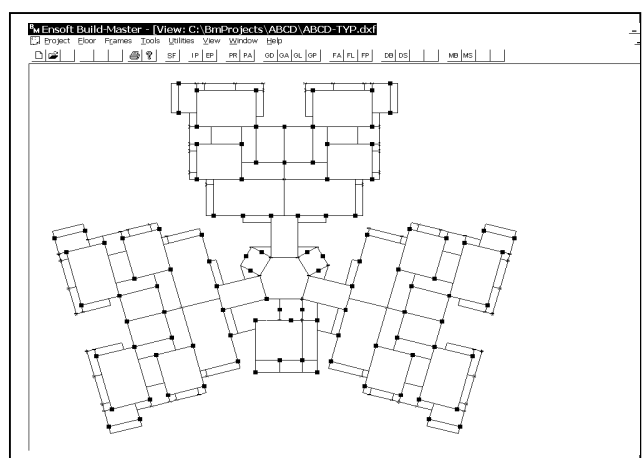
Steps for Space Frame Analysis with horizontal earthquake and wind loads :

1. Basic parameters for calculating Earthquake and wind loads as per code provisions are defined.
2. Seismic weight of each floor is calculated using Floor grid analysis results of that level.
3. Base Shear is calculated as per new formulae of IS 1893 – 2002 code.
4. Base shear is distributed along the building height and forces at each floor level are calculated.
5. Center of Gravity, Center of Rotation and Design eccentricity is worked out at each level.
6. Torsion moment due to eccentricity is calculated for two cases as per code.
7. Floor width and center of width is calculated at each level depending on floor dimensions.
8. Wind pressure and wind loads are calculated at each floor level.
9. Various load combinations are generated for Dead Loads, Imposed, Earthquake and Wind loads.
10. Frame analysis is carried out for the 3D model.

Frame Analysis Report for a Sample Building is listed below:



| Height (m) | Floor Description | Coordinate x | Coordinate y |
|------------|-------------------|-------------------|--------------|
| 34.000 | ABCD-RF | Roof | 2.900 |
| 31.100 | ABCD-TYP | 10th Floor | 2.900 |
| 28.200 | ABCD-TYP | 9th Floor | 2.900 |
| 25.300 | ABCD-TYP | 8th Floor | 2.900 |
| 22.400 | ABCD-TYP | 7th Floor | 2.900 |
| 19.500 | ABCD-TYP | 6th Floor | 2.900 |
| 16.600 | ABCD-TYP | 5th Floor | 2.900 |
| 13.700 | ABCD-TYP | 4th Floor | 2.900 |
| 10.800 | ABCD-TYP | 3rd Floor | 2.900 |
| 7.900 | ABCD-TYP | 2nd Floor | 2.900 |
| 5.000 | ABCD-TYP | 1st Floor | 2.900 |
| 2.100 | ABCD-PL | Plinth | 1.00 |
| 0.000 | Footing | <Height Unit: mt> | |



FRAME ANALYSIS REPORT (ENSOFT BUILD-MASTER Version 2005)

| PROJECT NAME | | : ABCD | | NUMBER OF LEVELS | | : 12 | | S E I S M I C W E I G H T | | |
|--------------|------------|-------------|------------------|-------------------|--------------|--------------|-----------------|---------------------------|--|--|
| LEVEL NO. | FLOOR CODE | HEIGHT (mt) | Ht.ABOVE FOOTING | FLOOR DESCRIPTION | TOTAL DL (T) | TOTAL IL (T) | IL % Percentage | REDUCED IL (T) | | |
| 1 | ABCD-PL | 2.100 | 2.100 | Plinth | 431.240 | 0.490 | 25.000 | 0.122 | | |
| 2 | ABCD-TYP | 2.900 | 5.000 | 1st Floor | 890.740 | 103.130 | 25.000 | 25.782 | | |
| 3 | ABCD-TYP | 2.900 | 7.900 | 2nd Floor | 890.740 | 103.130 | 25.000 | 25.782 | | |
| 4 | ABCD-TYP | 2.900 | 10.800 | 3rd Floor | 890.740 | 103.130 | 25.000 | 25.782 | | |
| 5 | ABCD-TYP | 2.900 | 13.700 | 4th Floor | 890.740 | 103.130 | 25.000 | 25.782 | | |
| 6 | ABCD-TYP | 2.900 | 16.600 | 5th Floor | 890.740 | 103.130 | 25.000 | 25.782 | | |
| 7 | ABCD-TYP | 2.900 | 19.500 | 6th Floor | 890.740 | 103.130 | 25.000 | 25.782 | | |
| 8 | ABCD-TYP | 2.900 | 22.400 | 7th Floor | 890.740 | 103.130 | 25.000 | 25.782 | | |
| 9 | ABCD-TYP | 2.900 | 25.300 | 8th Floor | 890.740 | 103.130 | 25.000 | 25.782 | | |
| 10 | ABCD-TYP | 2.900 | 28.200 | 9th Floor | 890.740 | 103.130 | 25.000 | 25.782 | | |
| 11 | ABCD-TYP | 2.900 | 31.100 | 10th Floor | 890.740 | 103.130 | 25.000 | 25.782 | | |
| 12 | ABCD-RF | 2.900 | 34.000 | Roof | 574.310 | 103.130 | 0.000 | 0.000 | | |

EARTHQUAKE DATA

Zone Factor Z = 0.160
 Damping Factor F = 1.000
 Importance Factor I = 1.000
 Building Height h = 34.000 mt
 Time Period = $0.09 * h / \text{sqrt}(d)$
 Seismic Zone : 3 Intensity : Moderate
 Damping Per : 5.000 %
 Response Reduction Factor R = 5.000
 Frame Type = 1.RC with Infill
 Soil Type = 1.Hard Soil
 Building Dimensions
 Base Dimensions in X BDimX : 37.644 mt
 Base Dimensions in Y BDimY : 30.686 mt

WIND DATA

Class of Structure : 1 All General Bldgs.(50 yr.)
 Basic Wind Speed : 44.000 mt/sec
 Building Class : B. Max Dim bet. 20 & 50 mt
 Terrain Category : 3. Closely spaced Less/Low obstructions
 Risk Coefficient K1 : 1.000
 Terrain size Factor K2 : 0.880 up to 10 mt
 Topography Factor K3 : 1.000
 Force Coeff. Cfx : 1.000
 Force Coeff. Cfy : 1.000
 Design Wind Speed : 38.720 mt/sec
 Bldg Height Bldg Ht h : 34.000 mt
 Plinth Height : 0.500 mt
 Parapet Height : 1.000 mt

BASE SHEAR CALCULATIONS

| PARAMETER | X Direction | Y Direction |
|-----------------------------------|-------------|-------------|
| Building Height | h 34.000 | 34.000 mt |
| Base Dimension at Plinth Level. | d 37.644 | 30.686 mt |
| Time Period in Sec. | T 0.499 | 0.552 sec |
| Avg. Response Acceleration Coeff. | Sa/g 2.005 | 1.810 |
| Design Horizontal Seismic Coeff. | Ah 0.032 | 0.029 |
| Seismic Weight of Building | w 10170.897 | 10170.897 T |
| Design Seismic Base Shear | Vb 326.291 | 294.597 T |

EARTHQUAKE FORCES APPLIED AT C.R. WITH ECCENTRICITY AT EACH FLOOR

| LEVEL NO. | FLOOR CODE | CENTRE OF GRAVITY CGX | CENTRE OF GRAVITY CGY | CENTRE OF ROTATION CRX | ROTATION CRY | ACTUAL ECCENTRICITY Esix | Esiy | 5% FLOOR WIDTH 0.05bx | 0.05by | FLOOR NODE NEAR NODE NO. | C.R. CordX | C.R. CordY | SPACE NODE |
|-----------|------------|-----------------------|-----------------------|------------------------|--------------|--------------------------|--------|-----------------------|--------|--------------------------|------------|------------|------------|
| 1 | ABCD-PL | 0.074 | -4.361 | 0.079 | -5.410 | 0.005 | -1.049 | 1.882 | 1.534 | 181 | 1.066 | -3.657 | 287 |
| 2 | ABCD-TYP | 0.008 | -4.351 | 0.079 | -5.410 | 0.071 | -1.059 | 1.882 | 1.534 | 181 | 1.066 | -3.657 | 629 |
| 3 | ABCD-TYP | 0.008 | -4.351 | 0.079 | -5.410 | 0.071 | -1.059 | 1.882 | 1.534 | 181 | 1.066 | -3.657 | 971 |
| 4 | ABCD-TYP | 0.008 | -4.351 | 0.079 | -5.410 | 0.071 | -1.059 | 1.882 | 1.534 | 181 | 1.066 | -3.657 | 1313 |
| 5 | ABCD-TYP | 0.008 | -4.351 | 0.079 | -5.410 | 0.071 | -1.059 | 1.882 | 1.534 | 181 | 1.066 | -3.657 | 1655 |
| 6 | ABCD-TYP | 0.008 | -4.351 | 0.079 | -5.410 | 0.071 | -1.059 | 1.882 | 1.534 | 181 | 1.066 | -3.657 | 1997 |
| 7 | ABCD-TYP | 0.008 | -4.351 | 0.079 | -5.410 | 0.071 | -1.059 | 1.882 | 1.534 | 181 | 1.066 | -3.657 | 2339 |
| 8 | ABCD-TYP | 0.008 | -4.351 | 0.079 | -5.410 | 0.071 | -1.059 | 1.882 | 1.534 | 181 | 1.066 | -3.657 | 2681 |
| 9 | ABCD-TYP | 0.008 | -4.351 | 0.079 | -5.410 | 0.071 | -1.059 | 1.882 | 1.534 | 181 | 1.066 | -3.657 | 3023 |
| 10 | ABCD-TYP | 0.008 | -4.351 | 0.079 | -5.410 | 0.071 | -1.059 | 1.882 | 1.534 | 181 | 1.066 | -3.657 | 3365 |
| 11 | ABCD-TYP | 0.008 | -4.351 | 0.079 | -5.410 | 0.071 | -1.059 | 1.882 | 1.534 | 181 | 1.066 | -3.657 | 3707 |
| 12 | ABCD-RF | 0.036 | -4.309 | 0.079 | -5.410 | 0.043 | -1.101 | 1.882 | 1.534 | 181 | 1.066 | -3.657 | 4049 |

TOTAL EARTHQUAKE LOAD APPLIED AT EACH FLOOR

| LEVEL NO. | FLOOR CODE | SPACE FRAME NODE NO. | FORCES IN X DIRECTION(T) | | | FORCES IN Z DIRECTION(T) | | |
|-----------|------------|----------------------|--------------------------|--------|----------|--------------------------|---------|----------|
| | | | EPX | TPX1 | TPX2 | EPZ | TPZ1 | TPZ2 |
| 1 | ABCD-PL | 287 | 0.145 | -0.006 | -0.375 | 0.131 | 0.248 | -0.246 |
| 2 | ABCD-TYP | 629 | 1.744 | -0.095 | -4.523 | 1.574 | 3.131 | -2.850 |
| 3 | ABCD-TYP | 971 | 4.353 | -0.237 | -11.289 | 3.930 | 7.818 | -7.117 |
| 4 | ABCD-TYP | 1313 | 8.135 | -0.443 | -21.098 | 7.345 | 14.611 | -13.301 |
| 5 | ABCD-TYP | 1655 | 13.091 | -0.713 | -33.951 | 11.819 | 23.511 | -21.402 |
| 6 | ABCD-TYP | 1997 | 19.219 | -1.047 | -49.844 | 17.352 | 34.517 | -31.422 |
| 7 | ABCD-TYP | 2339 | 26.521 | -1.445 | -68.782 | 23.945 | 47.632 | -43.361 |
| 8 | ABCD-TYP | 2681 | 34.996 | -1.907 | -90.762 | 31.596 | 62.852 | -57.215 |
| 9 | ABCD-TYP | 3023 | 44.644 | -2.433 | -115.784 | 40.307 | 80.180 | -72.990 |
| 10 | ABCD-TYP | 3365 | 55.465 | -3.022 | -143.848 | 50.077 | 99.615 | -90.682 |
| 11 | ABCD-TYP | 3707 | 67.459 | -3.676 | -174.954 | 60.906 | 121.156 | -110.291 |
| 12 | ABCD-RF | 4049 | 50.522 | -5.905 | -133.130 | 45.614 | 88.809 | -83.885 |

WIND LOADS APPLIED AT CENTRE OF WIDTH

| LEVEL NO. | FLOOR CODE | FLOOR MinX | FLOOR MaxX | COORDINATES MinY | COORDINATES MaxY | CENTRE OF WIDTH CWX | CENTRE OF WIDTH CWY | FLOOR NODE | FLOOR Node No. | NEARCW CordX | NEARCW CordY | SPACE NODE |
|-----------|------------|------------|------------|------------------|------------------|---------------------|---------------------|------------|----------------|--------------|--------------|------------|
| 1 | ABCD-PL | -18.822 | 18.822 | -19.079 | 11.607 | 0.000 | -3.736 | 180 | -1.066 | -3.657 | 286 | |
| 2 | ABCD-TYP | -18.822 | 18.822 | -19.079 | 11.607 | 0.000 | -3.736 | 180 | -1.066 | -3.657 | 628 | |
| 3 | ABCD-TYP | -18.822 | 18.822 | -19.079 | 11.607 | 0.000 | -3.736 | 180 | -1.066 | -3.657 | 970 | |
| 4 | ABCD-TYP | -18.822 | 18.822 | -19.079 | 11.607 | 0.000 | -3.736 | 180 | -1.066 | -3.657 | 1312 | |
| 5 | ABCD-TYP | -18.822 | 18.822 | -19.079 | 11.607 | 0.000 | -3.736 | 180 | -1.066 | -3.657 | 1654 | |
| 6 | ABCD-TYP | -18.822 | 18.822 | -19.079 | 11.607 | 0.000 | -3.736 | 180 | -1.066 | -3.657 | 1996 | |
| 7 | ABCD-TYP | -18.822 | 18.822 | -19.079 | 11.607 | 0.000 | -3.736 | 180 | -1.066 | -3.657 | 2338 | |
| 8 | ABCD-TYP | -18.822 | 18.822 | -19.079 | 11.607 | 0.000 | -3.736 | 180 | -1.066 | -3.657 | 2680 | |
| 9 | ABCD-TYP | -18.822 | 18.822 | -19.079 | 11.607 | 0.000 | -3.736 | 180 | -1.066 | -3.657 | 3022 | |
| 10 | ABCD-TYP | -18.822 | 18.822 | -19.079 | 11.607 | 0.000 | -3.736 | 180 | -1.066 | -3.657 | 3364 | |
| 11 | ABCD-TYP | -18.822 | 18.822 | -19.079 | 11.607 | 0.000 | -3.736 | 180 | -1.066 | -3.657 | 3706 | |
| 12 | ABCD-RF | -18.822 | 18.822 | -19.079 | 11.607 | 0.000 | -3.736 | 180 | -1.066 | -3.657 | 4048 | |

TOTAL WIND LOAD APPLIED AT EACH FLOOR

| LEVEL NO. | FLOOR CODE | HEIGHT | COEFF K2 | WIND SPEED Vz (mt/sec) | WIND PRESSURE Pz (T/mt2) | FORCES IN X DIRECTION FL. WIDTH | FORCES IN X DIRECTION LOAD SPACE NODE | FORCES IN Z DIRECTION FL. WIDTH | FORCES IN Z DIRECTION LOAD SPACE NODE | | |
|-----------|------------|--------|----------|------------------------|--------------------------|---------------------------------|---------------------------------------|---------------------------------|---------------------------------------|--------|------|
| 1 | ABCD-PL | 2.100 | 0.880 | 38.720 | 0.090 | 30.686 | 5.370 | 286 | 37.644 | 6.588 | 286 |
| 2 | ABCD-TYP | 5.000 | 0.880 | 38.720 | 0.090 | 30.686 | 8.009 | 628 | 37.644 | 9.825 | 628 |
| 3 | ABCD-TYP | 7.900 | 0.880 | 38.720 | 0.090 | 30.686 | 8.009 | 970 | 37.644 | 9.825 | 970 |
| 4 | ABCD-TYP | 10.800 | 0.940 | 41.360 | 0.103 | 30.686 | 9.144 | 1312 | 37.644 | 11.218 | 1312 |
| 5 | ABCD-TYP | 13.700 | 0.940 | 41.360 | 0.103 | 30.686 | 9.144 | 1654 | 37.644 | 11.218 | 1654 |
| 6 | ABCD-TYP | 16.600 | 0.980 | 43.120 | 0.112 | 30.686 | 9.942 | 1996 | 37.644 | 12.197 | 1996 |
| 7 | ABCD-TYP | 19.500 | 0.980 | 43.120 | 0.112 | 30.686 | 9.942 | 2338 | 37.644 | 12.197 | 2338 |
| 8 | ABCD-TYP | 22.400 | 1.030 | 45.320 | 0.123 | 30.686 | 10.955 | 2680 | 37.644 | 13.439 | 2680 |
| 9 | ABCD-TYP | 25.300 | 1.030 | 45.320 | 0.123 | 30.686 | 10.955 | 3022 | 37.644 | 13.439 | 3022 |
| 10 | ABCD-TYP | 28.200 | 1.030 | 45.320 | 0.123 | 30.686 | 10.955 | 3364 | 37.644 | 13.439 | 3364 |
| 11 | ABCD-TYP | 31.100 | 1.090 | 47.960 | 0.138 | 30.686 | 12.274 | 3706 | 37.644 | 15.058 | 3706 |
| 12 | ABCD-RF | 34.000 | 1.090 | 47.960 | 0.138 | 30.686 | 10.372 | 4048 | 37.644 | 12.724 | 4048 |

DESCRIPTION OF LOAD CASES

| | | |
|----|------|--|
| 1 | DL | Dead Load |
| 2 | IL | Imposed Load |
| 3 | RIL | Reduced Imposed Load |
| 4 | EPX | Earthquake Parallel to +ve X Direction |
| 5 | TPX1 | Torsion Case 1 for EQ +ve X Direction |
| 6 | TPX2 | Torsion Case 2 for EQ +ve X Direction |
| 7 | EPZ | Earthquake Parallel to +ve Z Direction |
| 8 | TPZ1 | Torsion Case 1 for EQ +ve Z Direction |
| 9 | TPZ2 | Torsion Case 2 for EQ +ve Z Direction |
| 10 | WPX | wind Parallel to +ve X Direction |
| 11 | WPZ | wind Parallel to +ve Z Direction |

LOAD COMBINATIONS FOR DL, IL & WIND LOADS

| | |
|---------------------------|-----------------------------|
| 1.5 DL + 1.5 IL | (1 COMBINATION OF DL + IL) |
| 1.5 DL ± 1.5 WPX | |
| 1.5 DL ± 1.5 WPZ | |
| 0.9 DL ± 1.5 WPX | |
| 0.9 DL ± 1.5 WPZ | |
| 1.2 DL + 1.2 IL ± 1.2 WPX | |
| 1.2 DL + 1.2 IL ± 1.2 WPZ | (12 COMBINATIONS OF WIND) |

(Horizontal forces in -ve Axis direction are of same magnitude of +ve Axis but with -ve sign.)

Depending on whether Building is Orthogonal or Not and Earthquake Eccentricity is considered or Not Program generates following **Load Combinations** with Earthquake Loads:

ORTHOGONAL BUILDING WITHOUT ECCENTRICITY

1.5 DL ± 1.5 EPX
 1.5 DL ± 1.5 EPZ
 0.9 DL ± 1.5 EPX
 0.9 DL ± 1.5 EPZ
 1.2 DL + 1.2 RIL ± 1.2 EPX
 1.2 DL + 1.2 RIL ± 1.2 EPZ
 Earthquake combinations = 12
 Total Load Combinations = 25

NON ORTHOGONAL BUILDING WITHOUT ECCENTRICITY

1.5 DL ± 1.5 EPX ± 0.45 EPZ
 1.5 DL ± 1.5 EPZ ± 0.45 EPX
 0.9 DL ± 1.5 EPX ± 0.45 EPZ
 0.9 DL ± 1.5 EPZ ± 0.45 EPX
 1.2 DL + 1.2 RIL ± 1.2 EPX ± 0.36 EPZ
 1.2 DL + 1.2 RIL ± 1.2 EPZ ± 0.36 EPX
 Earthquake combinations = 24
 Total Load Combinations = 37

ORTHOGONAL BUILDING WITH ECCENTRICITY

1.5 DL ± 1.5 EPX ± 1.5 TPX1
 1.5 DL ± 1.5 EPX ± 1.5 TPX2
 1.5 DL ± 1.5 EPZ ± 1.5 TPZ1
 1.5 DL ± 1.5 EPZ ± 1.5 TPZ2
 0.9 DL ± 1.5 EPX ± 1.5 TPX1
 0.9 DL ± 1.5 EPX ± 1.5 TPX2
 0.9 DL ± 1.5 EPZ ± 1.5 TPZ1
 0.9 DL ± 1.5 EPZ ± 1.5 TPZ2
 1.2 DL + 1.2 RIL ± 1.2 EPX ± 1.2 TPX1
 1.2 DL + 1.2 RIL ± 1.2 EPX ± 1.2 TPX2
 1.2 DL + 1.2 RIL ± 1.2 EPZ ± 1.2 TPZ1
 1.2 DL + 1.2 RIL ± 1.2 EPZ ± 1.2 TPZ2
 Earthquake Combinations = 48
 Total Load Combinations = 61

NON-ORTHOGONAL BUILDING WITH ECCENTRICITY

1.5 DL ± 1.5 EPX ± 1.5 TPX1 ± 0.45 EPZ ± 0.45 TPZ1
 1.5 DL ± 1.5 EPX ± 1.5 TPX2 ± 0.45 EPZ ± 0.45 TPZ2
 1.5 DL ± 1.5 EPZ ± 1.5 TPZ1 ± 0.45 EPX ± 0.45 TPX1
 1.5 DL ± 1.5 EPZ ± 1.5 TPZ2 ± 0.45 EPX ± 0.45 TPX2
 0.9 DL ± 1.5 EPX ± 1.5 TPX1 ± 0.45 EPZ ± 0.45 TPZ1
 0.9 DL ± 1.5 EPX ± 1.5 TPX2 ± 0.45 EPZ ± 0.45 TPZ2
 0.9 DL ± 1.5 EPZ ± 1.5 TPZ1 ± 0.45 EPX ± 0.45 TPX1
 0.9 DL ± 1.5 EPZ ± 1.5 TPZ2 ± 0.45 EPX ± 0.45 TPX2
 1.2 DL + 1.2 RIL ± 1.2 EPX ± 1.2 TPX1 ± 0.36 EPZ ± 0.36 TPZ1
 1.2 DL + 1.2 RIL ± 1.2 EPX ± 1.2 TPX2 ± 0.36 EPZ ± 0.36 TPZ2
 1.2 DL + 1.2 RIL ± 1.2 EPZ ± 1.2 TPZ1 ± 0.36 EPX ± 0.36 TPX1
 1.2 DL + 1.2 RIL ± 1.2 EPZ ± 1.2 TPZ2 ± 0.36 EPX ± 0.36 TPX2
 Earthquake Combinations = 192
 Total Load Combinations = 205